

# Failure Probability of Mining Slope Recommendation at Asphalt Mine, Buton, South East Sulawesi

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**Abstract:** Ministerial Decree No 1827 K/30/MEM/208 states that mine geotechnics should include mining slope's probability of failure (PoF) value, beside safety factor (SF), to express a degree stability. PoF tolerances are associated with the risk of consequence when the slope failures. This regulation conforms with geological uncertainty commonly encountered at asphalt mining industry. Overall asphalt mining highwall that is constituted by layers of sedimentary rocks subject to variation of its geotechnical parameters value. Required rock characteristics of Mohr-Coulomb's failure Criteria, i.e. cohesion and internal friction angle were statistically analysis to map their probability distributions. Numerical analysis is then carried out to determine slope's PoF for several slope dips and then followed with correlation and regression analysis. Correlation analysis indicates that PoF value rises as the slope dips steeper. Regression analysis results that overall slope dip recommendations are 48°, 55°, and 62.5° for high, moderate, and low failure consequence respectively. Considering mineable reserve conservation, steeper slope would not be recommended.

**Keywords:** Mining Slope, Asphalt Mine

## 1. Introduction

Surface mining operation always face slope stability problems. So do asphalt mine that operates in Buton, South East Sulawesi, Indonesia. Mining slope in asphalt mine is constituted by layers of sedimentary rocks, such as mudstone, siltstone, and sandstone beside asphalt seam as can be seen in sectional area of asphalt mining slope at Figure 1 (Duwila, 2017). Since asphalt exploitation is still advancing, recommendation of mining slope geometry is needed to ensure the safety working environment and the production continuity.

Recommendation of slope geometry is based on its stability quantification. Beside safety factors (SF), attention to probability of failure (PoF) arises. University research about probability of failure is unceasing that is indicated by Doctorate University of Alberta in Canada (El-Ramy, Morgensten and Cruden, 2002). Slope geometry recommendation based on probability of failure utilizes variability of rock mass characteristics that constitute the slope. Probability of Failure can be assessed by seeing SF value distribution that is shaped as the result of combinations of rock mass engineering characteristics.

Ministerial Decree issued by Ministry of Energy and Mineral Resources of Republic of Indonesia No 1827 K/30/MEM/2018 about Guideline of Good Mining Practice, article about feasibility study states that mining geotechnics should include probability of failure value in their documents beside the design of the slope models. So, this article is made to give the slope geometry recommendation that can satisfy the criteria on the regulation.

## 2. Methodology

Methodology of the research can be seen as a flow chart in Figure 2. Rock mass characteristics are collected laboratory test conducted and summarized for a thesis by Duwila (2017). Topography map, rocks distribution, cross sections, and water table elevations are also collected from company's archives.

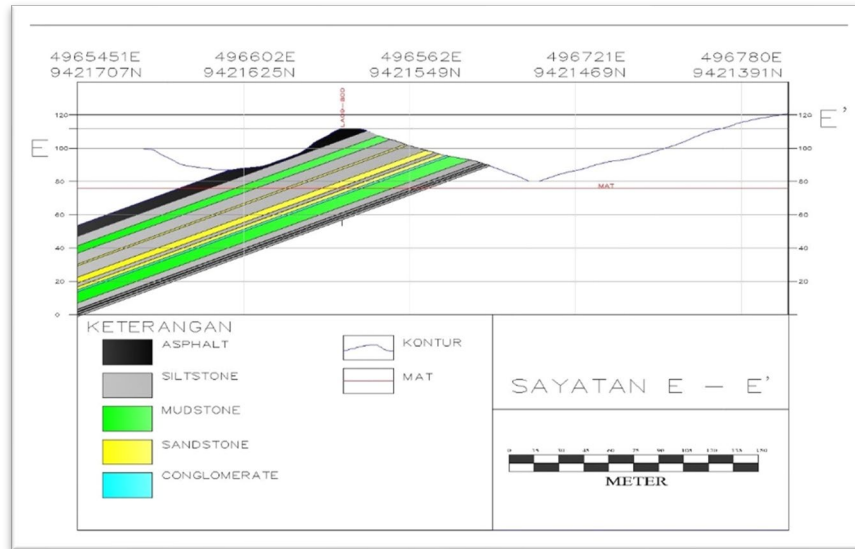
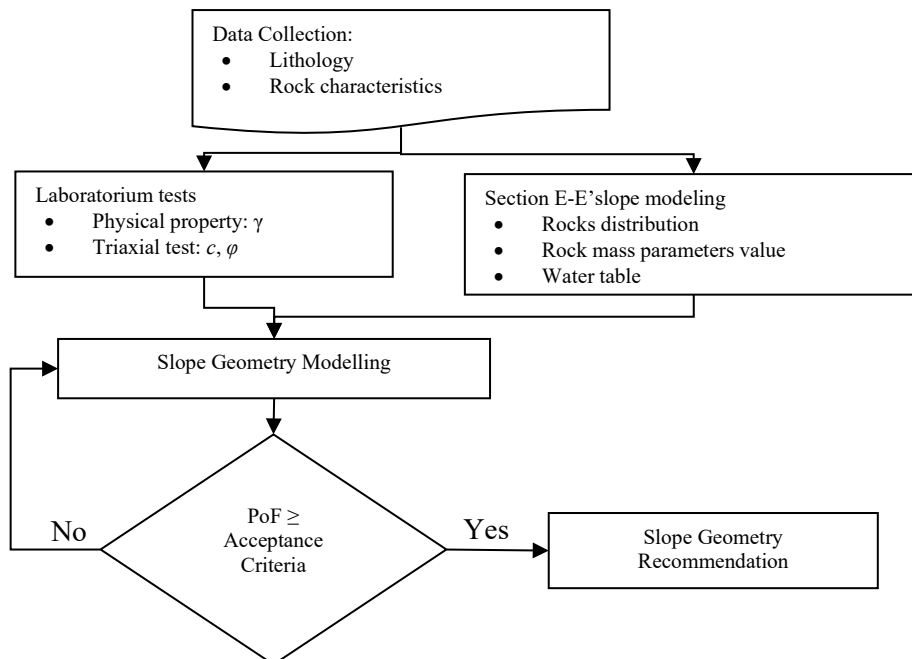


Fig. 1. Cross section E-E' of asphalt mine, Buton, South East Sulawesi



**Fig. 2.** Analysis flow chart

Safety Factor and Probability of Failure are calculated using Mohr-Coulomb's failure criteria. The slope geometry that is recommended is one whose Probability of Failure satisfies acceptance criteria stated in Ministerial Decree No 1827 K/30/MEM/2018 (Table 1). Acceptance criteria are based on consequence of failure. Consequence of failure can be interpreted as how high the cost would be paid to remediate the impact. Should the cost higher, the consequence also higher and it is indicated by the PoF with lower tolerance. During mining project, consequence of failure changes depending on the slope importance. Working face and hauling road is considered more important than abandoned or disposal slope since fatality, injury, damage of equipment and closed hauling access could occur.

The variation value of Mohr-Coulomb's rock parameters i.e. cohesion ( $c$ ) and internal friction angle ( $\Phi$ ) are needed. Their values are processed statistically to produce their mean, minimal, maximum, and standard deviation value. Those values would be entered as input data into the numerical program to calculate the probability of failure. Combinations of value of two parameters yields a variety of FS values. Ratio of FS values under 1 with all possible FS values is known as PoF. Illustration that depicts SF and PoF from two variables cohesion and internal friction angle can be seen in Figure 3.

Overall slope stability would be evaluated at several value of its dip. Recommended slope geometry is the ones that give PoF lower than acceptance criteria stated in the Ministerial Decree. As the consequence of failure is a subject to change, more than one slope geometries could be suggested for one slope section. Graphical output would be presented and regression formula would be provided to understand the maximum limit of slope geometry for each level of consequence of failure.

### *1.1. Slope model and failure mode approach*

Slope model is based on sectional area of mining pit Section E-E'. Mining slope is constituted by prementioned rock layers that dip gently which can be seen at Figure 1. High wall of the mining pit would dip against the layer's dip direction. SF and FoF would be determine on the highwall side of the mining slope.

Considering there are bedding planes between each rock layers, rock slides as plane failure with those bedding plane act as weakness plane where the rocks sliding on. This type of non-circular failure surface is taken as the failure mode to calculate the SF and PoF values of the asphalt mining overall slope.

### 1.2. Rock characteristics

For analitical purpose, laboratorium test of rock charateristics are needed. Physical properties test and direct shear test was carried out to determine density ( $\gamma$ ), cohesion ( $c$ ), and internal friction angle ( $\Phi$ ). There are four kinds of rock to be tested, e.g. sandstone, mudstone, siltstone, and asphalt. The laboratorium tests result of geotechnical parameters mean value can be seen at Table 2.

As stated before, the variety of rock parameter values are processed statistically. Two parameters from Mohr-Coulomb's failure criteria, cohesion ( $c$ ) and internal friction angle ( $\Phi$ ) values of each rock are modeled so they have some required descriptive statistics parameters such as mean value, standard deviation, minimum and maximum value. To give estimation of parameter variability values around mean symmetrically, not skewed toward the bigger or the lesser value, both parameters are considered normally distributed. Rocks density is left fixed at only one value, as it is not a Mohr-Coulomb's failure criteria variable.

## 3. Results

Results of numerical calculation using Mohr-Coulomb's failure criteria can be seen at Figure 3 – Figure 7 and are summarized in Table 3. Numerical calculation indicates that the PoF value rises as the overall slope angle decreases. At overall slope angle  $80^\circ$ , PoF is 46.1%. Meanwhile, at overall slope angle  $70^\circ$ , PoF is 59.7%. Both of these values of PoF are above acceptance level even for low risk consequence of failure. Thus, overall slope that dips  $70^\circ$  or greater is not recommended for section E-E'.

Decreasing overall slope angle to  $60^\circ$ ,  $50^\circ$ , and  $40^\circ$ , the corresponding PoF are 19.9%, 5.5%, and 1.7% respectively. From these three results, one can see that for low risk consequence, overall slope with dips  $60^\circ$  is acceptable. Meanwhile for medium risk consequence, overall slope with dips  $50^\circ$  is acceptable, and for high risk consequence, overall slope with dips  $40^\circ$  is acceptable.

**Table 1.** PoF classification by slope's consequence of failure (Ministry of Energy and Mineral Resources of Republic of Indonesia No 1827 K/30/MEM/2018)

Slope type	Consequences of Failure	Acceptance Criteria		
		Static Safety Factor (SF)	Dinamic Safety Factor (SF)	Probability of Failure (PoF)
Single Slope	Medium to High	1.1	N/A	25 – 50 %
	Low	1.15 – 1.2	1.0	25 %
Inter-ramp	Moderate	1.2 – 1.3	1.0	20 %
	High	1.2 – 1.3	1.1	10 %
Overall Slope	Low	1.2 – 1.3	1.0	15 – 20 %
	Moderate	1.3	1.05	10 %
	High	1.3 – 1.5	1.1	5 %

**Table 2.** Mean Value of Rock Mass Characteristics

	Density (MN/m <sup>3</sup> )	$\sigma_{ci}$ (MPa)	$c$ (MPa)	$\Phi$ (°)	$\sigma_t$ (MPa)
<i>Asphalt</i>	0,0156	0,575847	0,080712	26,3176	-0,00175
<i>Mudstone</i>	0,0192	3,25	0,075424	17,4773	-0,00903
<i>Siltstone</i>	0,0157	161,688	0,105107	28,2864	-0,01208
<i>Sandstone</i>	0,0166	235,244	0,180389	38,9621	-0,01306
<i>Siltstone</i>	0,0176	210,915	0,145602	31,5389	-0,02471
<i>Mudstone</i>	0,0196	345,999	0,114428	23,6302	-0,03441

To optimize the mineable reserve, slope geometries that give boundary value of PoF in each level of risk are needed to be determined. Slope angles at each level of risk consequence of failure limit can be calculated by correlation and regression analysis (Figure 8). Correlation analysis indicated that slope dip has positive relation with PoF value and the regression model could describe 94.05% of parameter value variations.

Using PoF values from each modeled overall slope angles, power regression depicts that PoF is a function of overall slope angle which satisfies  $PoF = 6.10^{-9}$  (Overall Slope Angle)<sup>5.302</sup>. Graphical output from regression and analysis can be seen at Figure 9.

Roof limit of PoF are 5%, 10%, and 20% for high, moderate, and low consequence of failure. When risks of failure consequence are high, the asphalt mining overall slope could be constructed to dip 48°. For moderate risks, slope dip could be steeper up to 55°. For low risks, mining overall slope could be constructed to dip 62.5°. Any steeper, PoF value would pass beyond acceptance criteria, thus unrecommendable.

#### 4. Conclusions

Should consequence of failure higher, tolerance of PoF value would be lower. Asphalt mining Slope with high risks of failure consequence needs to have slope dip only up to 48°. For moderate risks, slope dip could be steeper up to 55°. While For low risks, mining overall slope could be steeper up to 62.5°. Mining with slope dip any steeper would not be acceptable in optimization context.

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